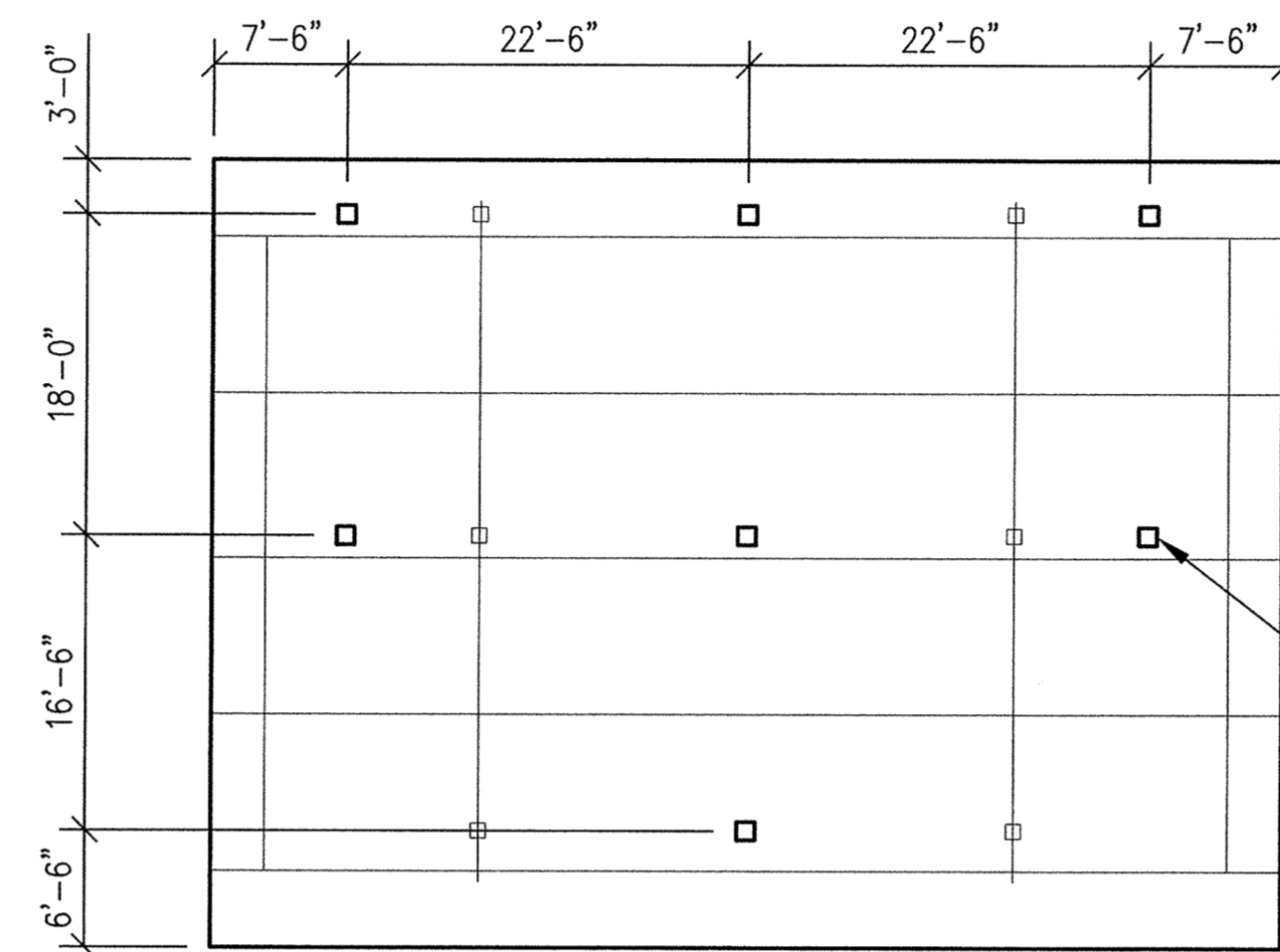
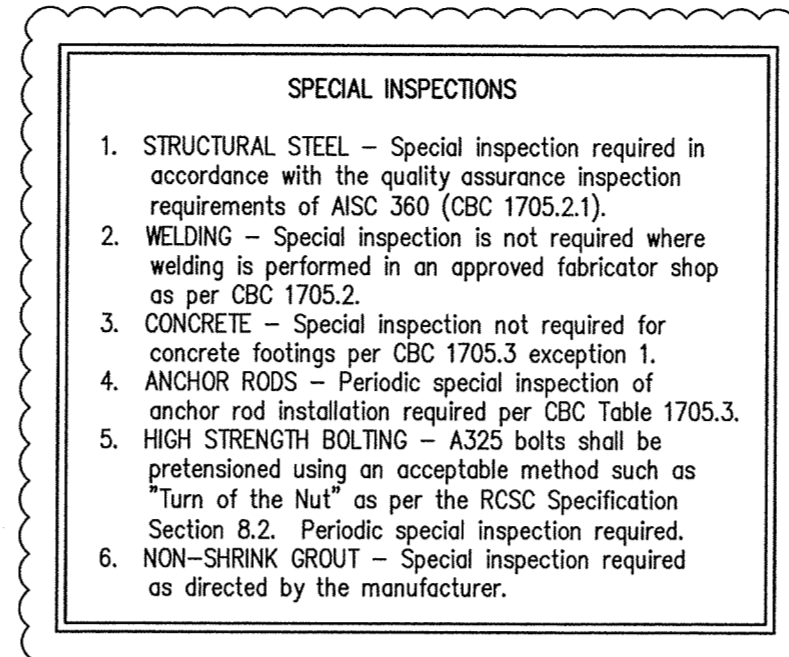
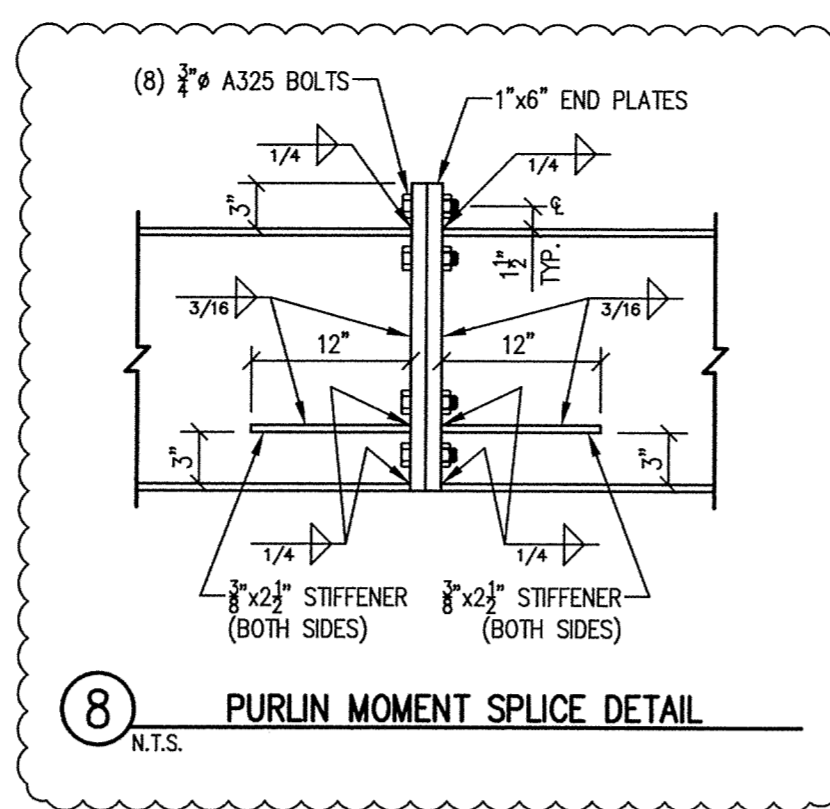
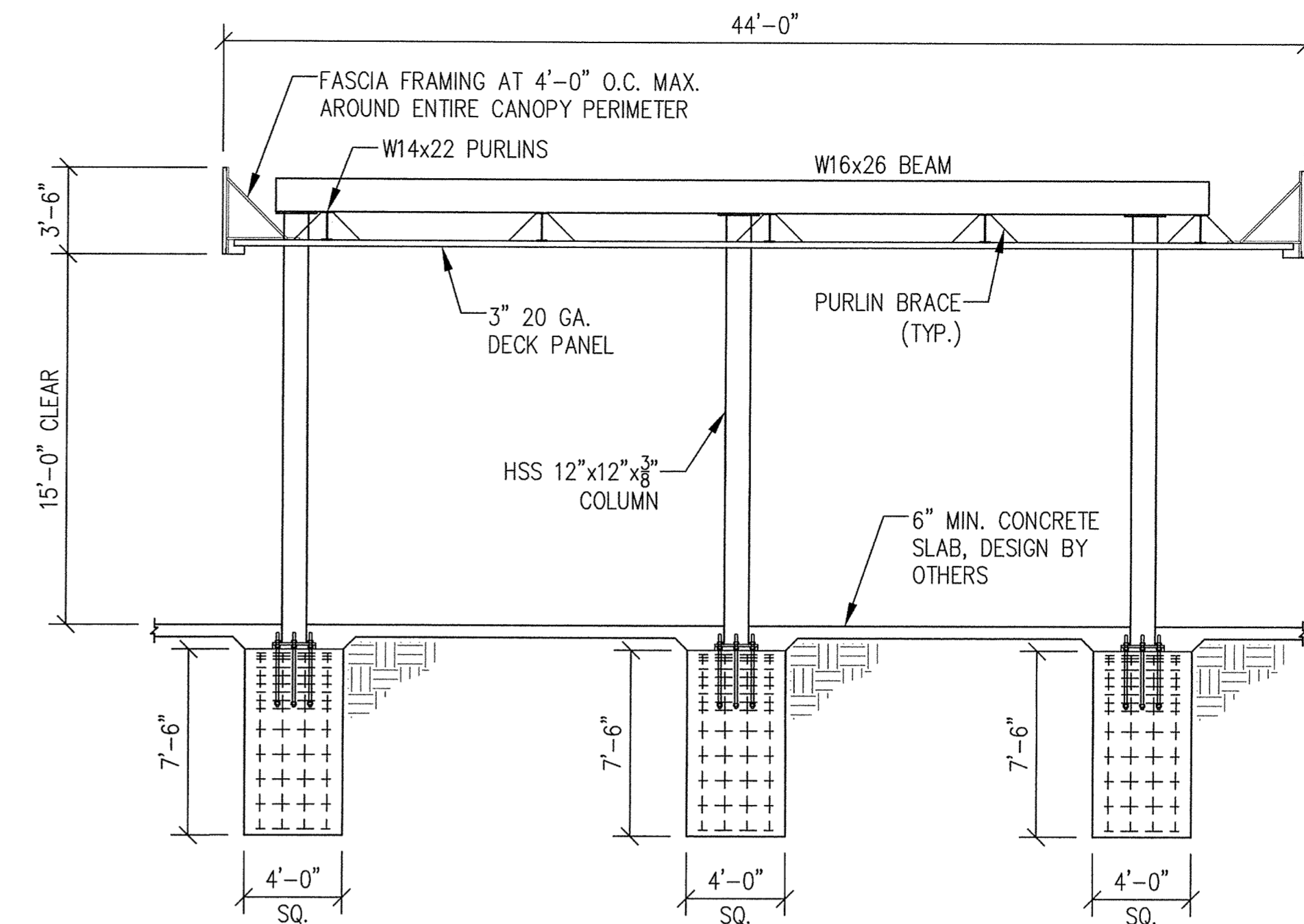


FRAMING PLAN
SCALE: 1/8"=1'-0"



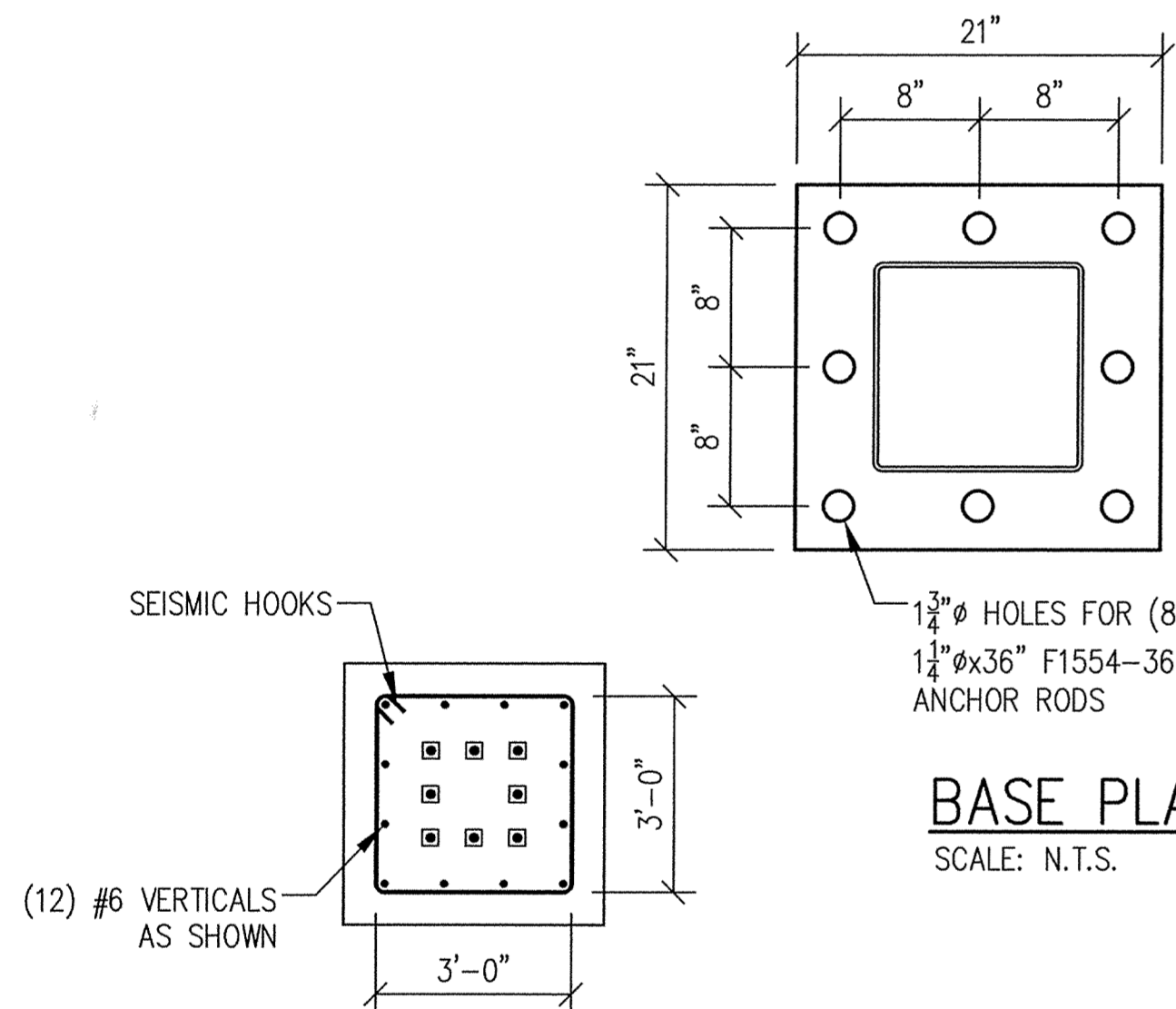
LIGHTING SCHEME
SCALE: 3/32"=1'-0"



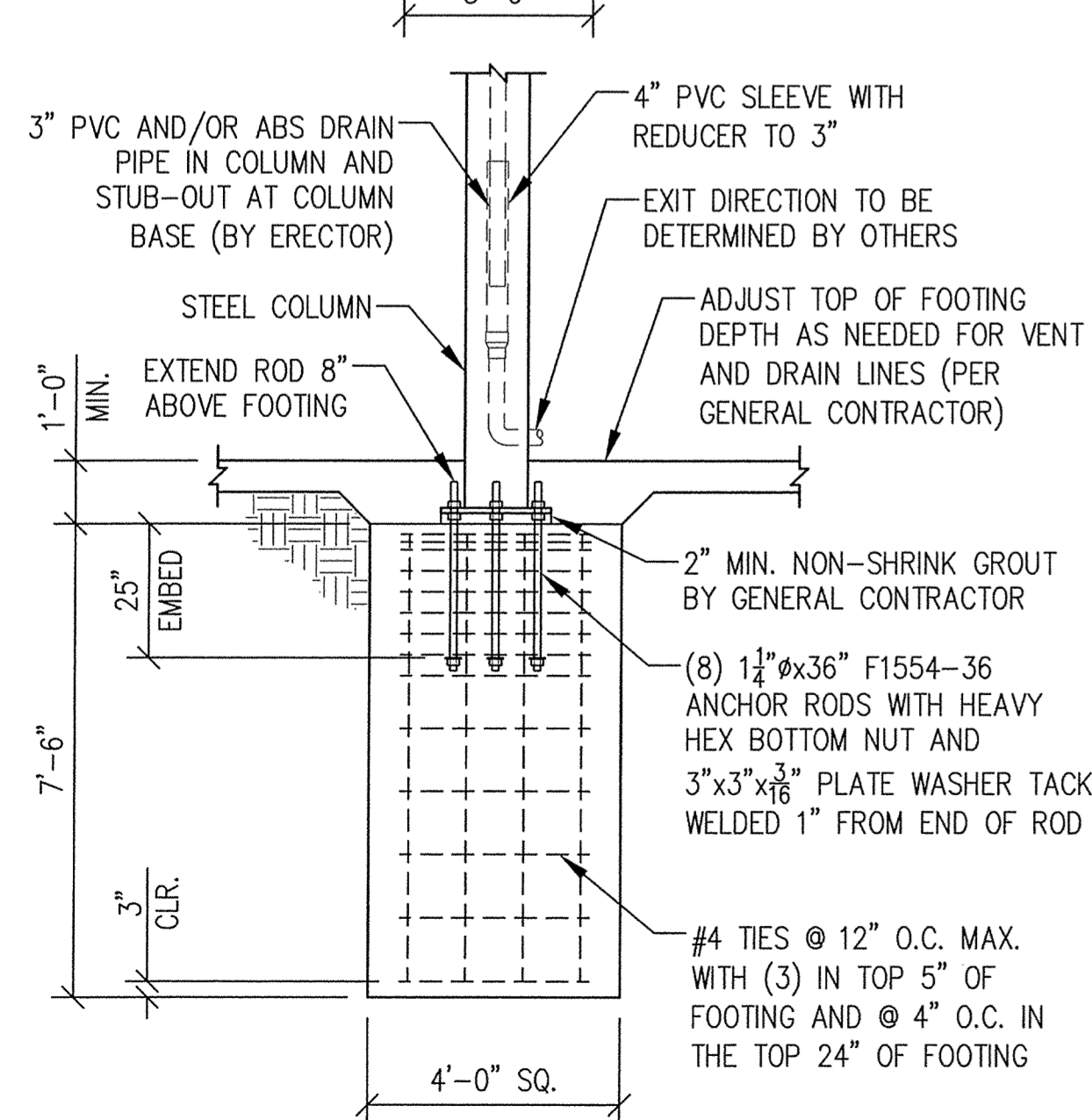
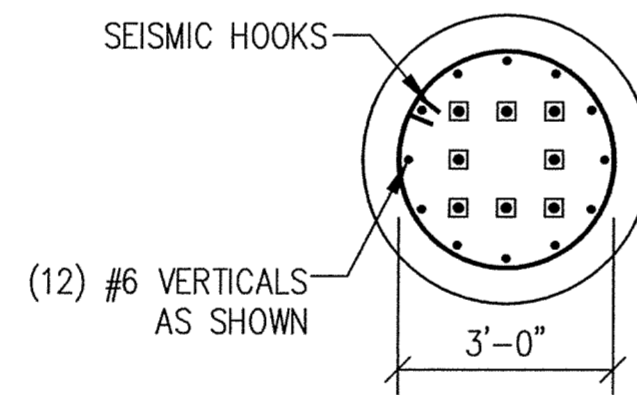
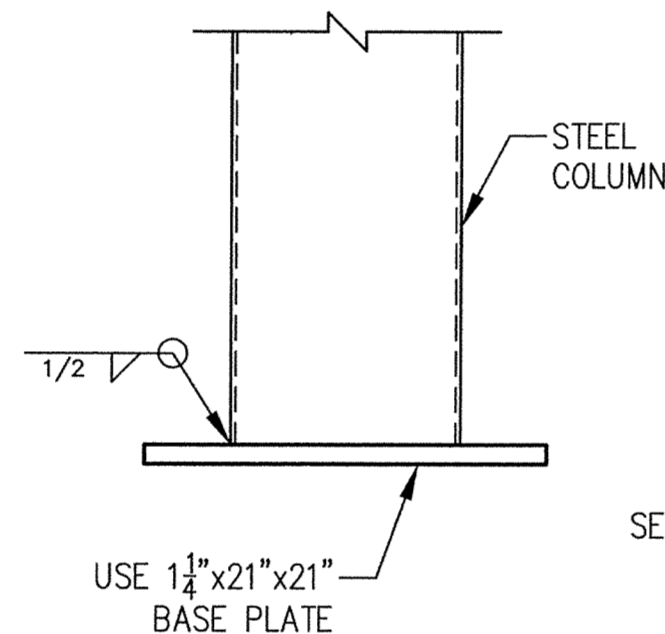
SECTION/ELEVATION
SCALE: 3/16"=1'-0"

SEISMIC FORCE RESISTING SYSTEM (SFRS) INFO

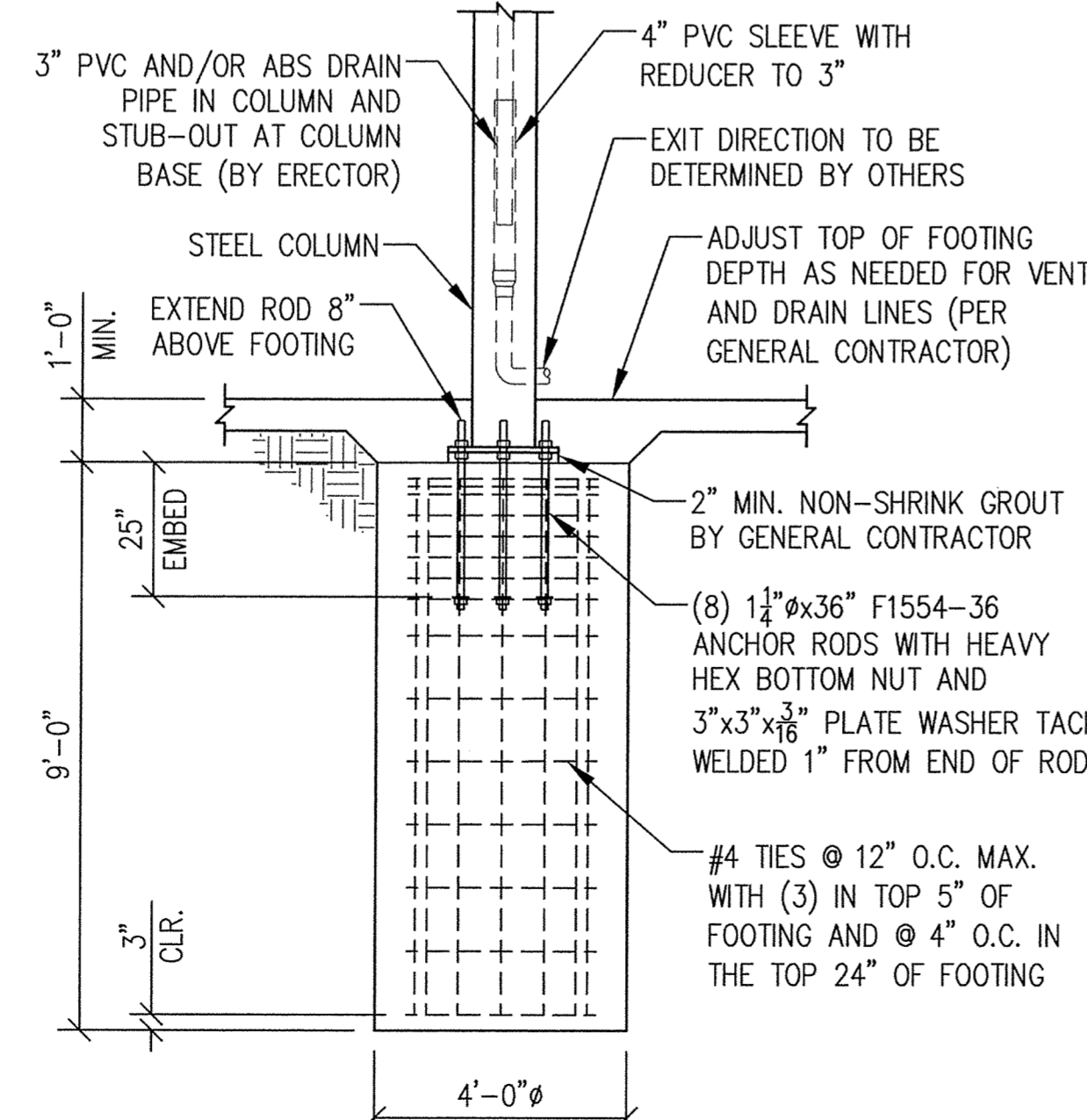
A. Refer to AISC 341-10 Section A4 and ASCE 7-10.
B. Designation of the SFRS: C.2. Steel ordinary cantilever column systems (ASCE 7-10 Table 12.2-1). See AISC 341-10 Section E5 (OCCS).
C. R = 1.25, Cs = 1.63
D. Analysis Procedure = Equivalent Lateral Force
E. Members and Connections that are part of the SFRS:
a. Columns
b. Column Base Connection
c. Footings
F. Protected Zones - N.A.
G. See details and notes for connection configurations, material specifications, and sizes.
H. Weld filler materials for welds part of the SFRS:
a. Yield Strength = 58 ksi min.
b. Tensile Strength = 70 ksi min.
c. Elongation = 22% min.
d. CVN Toughness = 20 ft-lb min. @ 0° F
See details and notes for other welding requirements.
J. Demand Critical Welds - N.A.
K. Lowest Anticipated Service Temperature - 0° F



BASE PLATE DETAIL
SCALE: N.T.S.



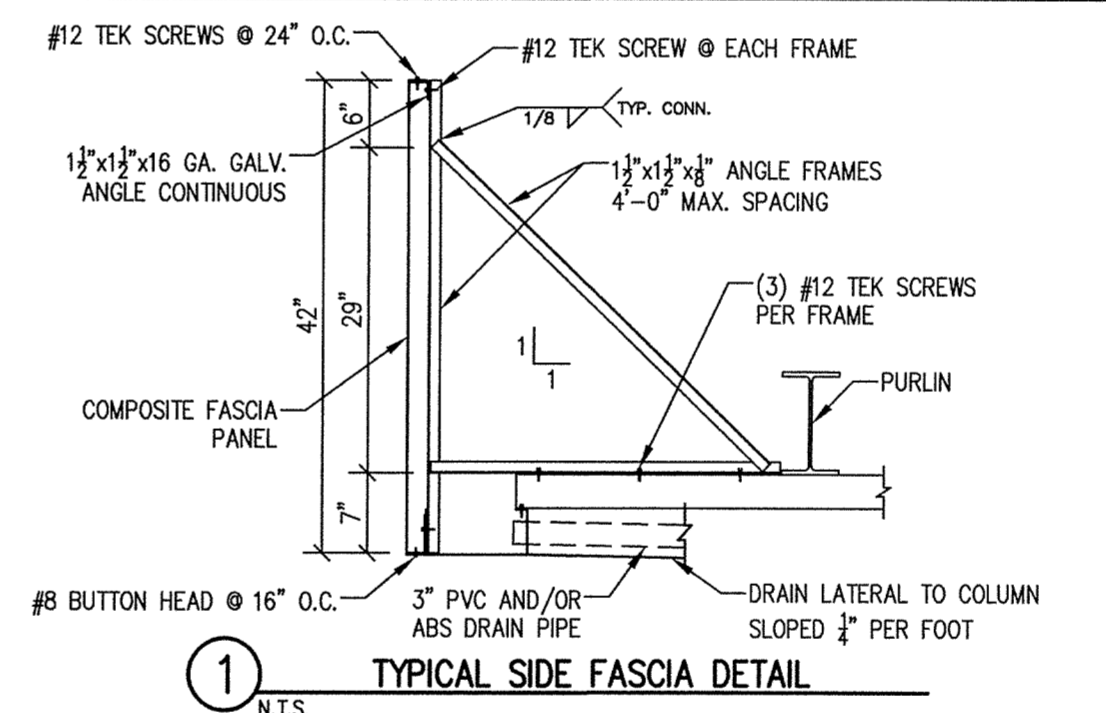
SQUARE FOOTING DETAIL
SCALE: 3/8"=1'-0"



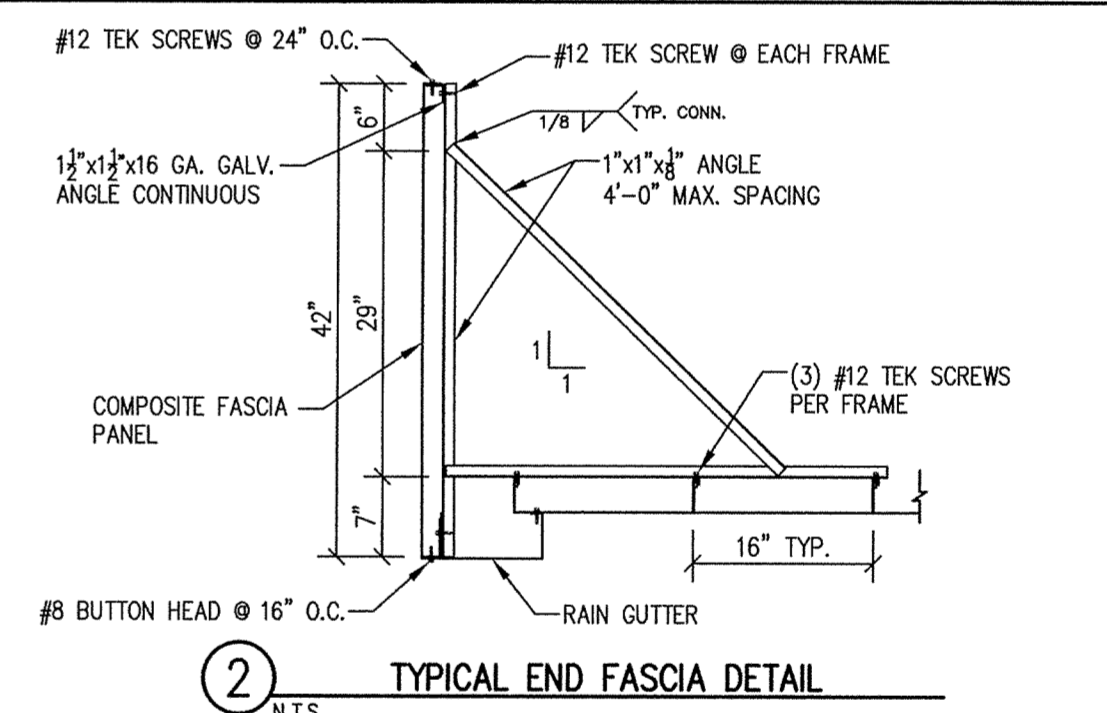
OPTIONAL ROUND FOOTING DETAIL
SCALE: 3/8"=1'-0"

STRUCTURAL NOTES:

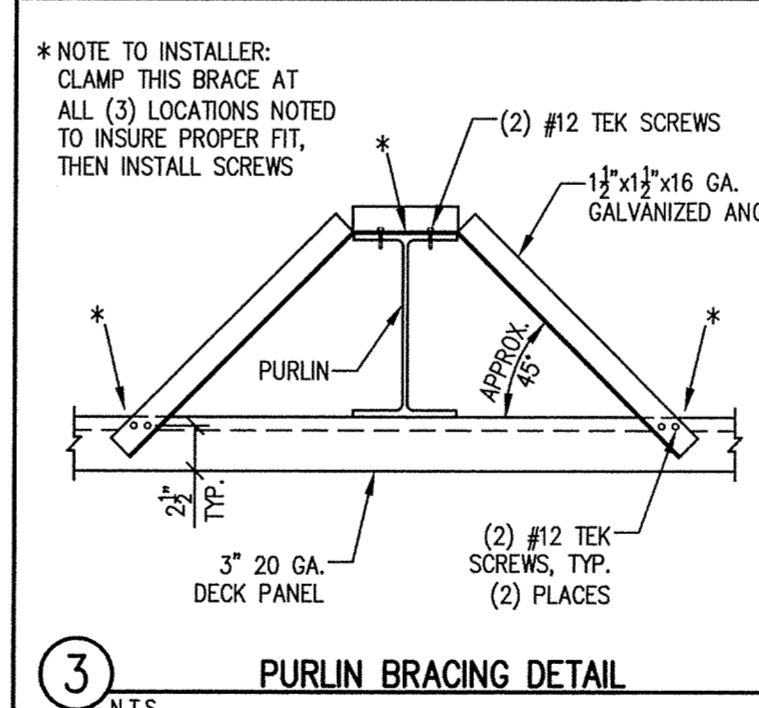
- A. GENERAL**
1. The contractor shall verify all conditions and dimensions at the site.
 2. Observation visits to the site by the design engineer shall neither be construed as inspection nor approval of construction.
 3. During and after construction, builder and/or owner shall keep loads on the structure within limits of design loads.
 4. Typical details and sections shall apply where specific details are not shown.
- B. 2013 CALIFORNIA BUILDING CODE DESIGN CRITERIA**
- A. Roof Live Load 20 psf
B. Canopy Dead Load 9 psf
C. Ground Snow Load - N.A.
D. Ultimate Design Wind Speed V_{ult} 115 mph
Risk Category II
Exposure C
Component & Cladding See ASCE 7-10 Chapter 30
E. Earthquake Design Data
Importance Factor I_e 1.0
 S_a 3.058g S_{pg} 2.038g
 S_1 1.186g S_{pg} 1.186g
Site Class D
Seismic Design Category E
Seismic Force Resisting System C.2. Steel ordinary cantilever column systems
- Design Base Shear 38.74 kips
Seismic Response Coefficient C_s 1.63
Response Modification Factor R 1.25
- C. FOUNDATION**
1. Allowable Soil Bearing Pressure: 1500 psf (not tested)
 2. Design Lateral Soil Pressure: 200 psf/ft of depth
 3. All footings shall be placed on and against undisturbed soil or granular fill mechanically compacted to not less than 95% of modified proctor density (ASTM D-1557).
- D. CONCRETE AND REINFORCEMENT**
1. Concrete Footings (Exposure Classes F0, S0, PO, CO) f'_c =3000 p.s.i., Type II cement, w/c ratio=0.45, Air Content=5% (±1.5%)
 2. Slabs on grade (Exposure Classes F1, S0, PO, CO) Type II cement, f'_c =3500 p.s.i.
 3. All concrete has been designed using f'_c =2500 p.s.i. Special Inspection not required unless noted otherwise.
 4. Rebar: ASTM A615 Grade 60.
 5. Grout under steel base plates shall be non-shrink grout complying with ASTM C1107, f'_c =5000 p.s.i., and installed per manufacturer's recommendations.
 6. The general contractor is responsible for locating top of footing elevations as required for underground plumbing and electrical.
- E. STRUCTURAL STEEL**
1. Structural steel shall be fabricated and erected in accordance with the latest edition of the following:
a. AISC "Specifications for the Design Fabrication and Erection of Structural Steel for Buildings."
b. AISC "Code of Standard Practice" excluding the following sections: 4.4, 4.4.1, 4.4.2, and 7.15.
c. AISC "Specifications for the Design of Cold Formed Structural Members."
 2. Structural steel shall conform to ASTM A36 for plates and angles, ASTM A500 grade B F_y = 42 k.s.i. min. for HSS pipes, and ASTM A992 grade 50 for all beams and purlins.
 3. Cold-formed steel tubing shall conform to ASTM A500, grade B.
 4. All welding shall be performed by certified welders in accordance with the American Welding Society using E-70xx electrodes.
 5. On all bolted connections, use ASTM A307 bolts unless otherwise specified (no special inspection required).
 6. Metal roof deck shall be 20 ga. ASTM A653 grade 40.



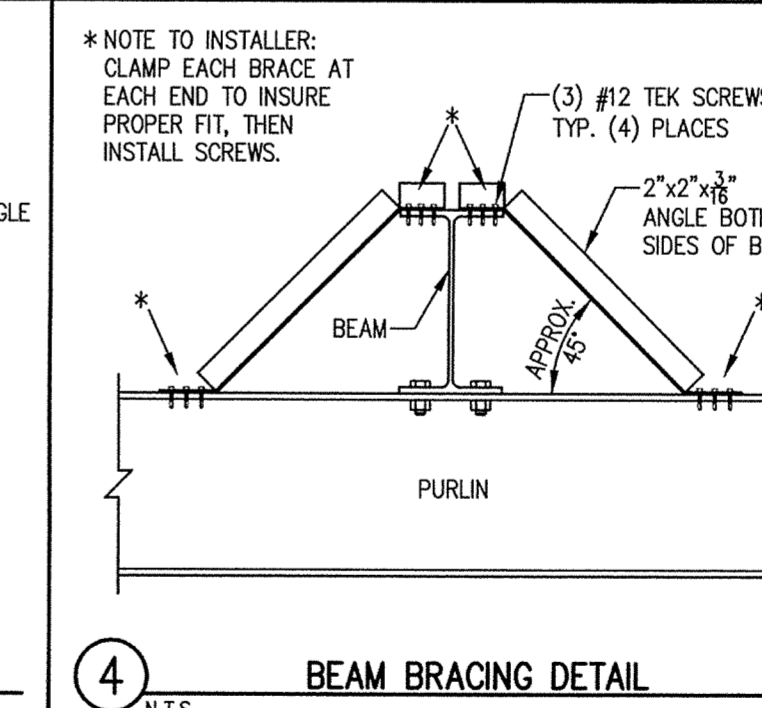
1 TYPICAL SIDE FASCIA DETAIL
N.T.S.



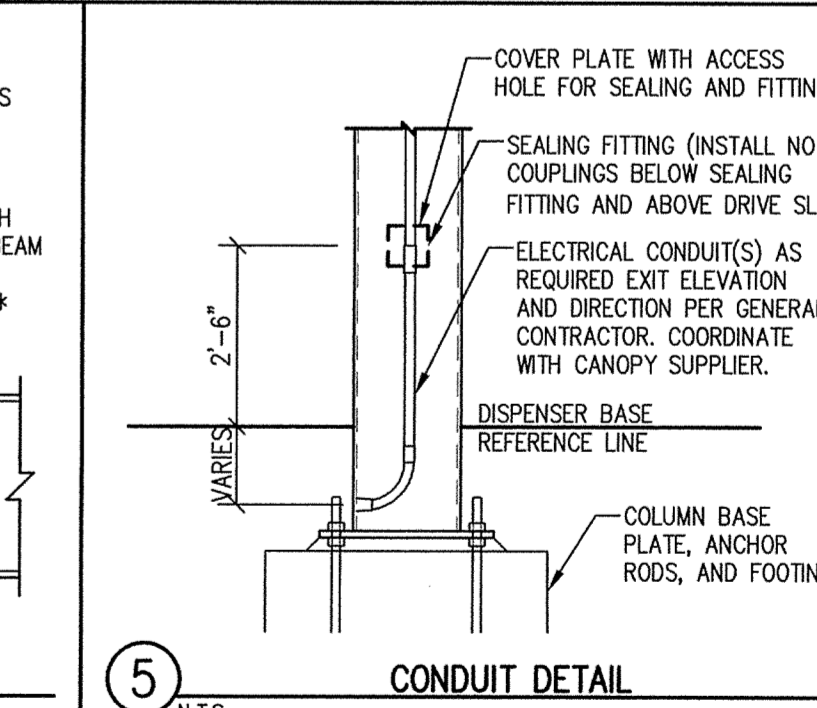
2 TYPICAL END FASCIA DETAIL
N.T.S.



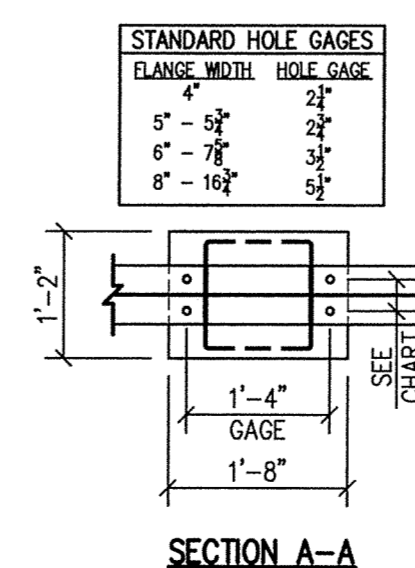
3 PURLIN BRACING DETAIL
N.T.S.



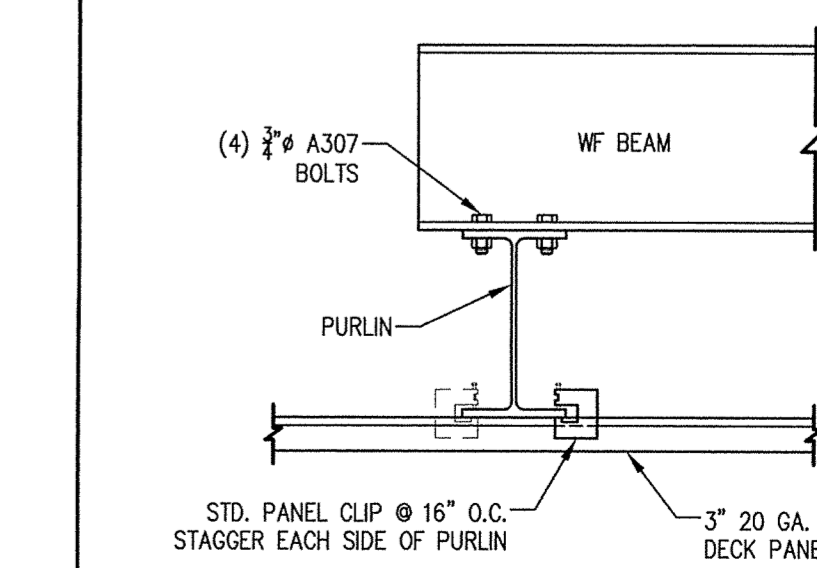
4 BEAM BRACING DETAIL
N.T.S.



5 CONDUIT DETAIL
N.T.S.



6 COLUMN/BEAM CONNECTION DETAIL
N.T.S.



7 BEAM/PURLIN/DECK CONNECTION DETAIL
N.T.S.

PROJECT: **HUMBOLDT TRANSIT AUTHORITY FUELING FACILITY PROJECT**

LOCATION: **133 V STREET, EUREKA, CALIFORNIA**

CANOPY SIZE: **44' x 60'**

MOUNTAIN VIEW ENGINEERING, INC.

345 No. Main St., Suite A
Brigham City, Utah 84302

PRECISION CANOPY

1497 West 40 South
London, Utah 84042

PLAN	ISSUE DATES	DESCRIPTION:
DATE:	BY:	FOR PERMIT
1-20-16	C.R.H.	FOR PERMIT
3-10-16	C.R.H.	ADDED PURLIN MOMENT SPLICE



MAR 10 2016

SHEET NUMBER:

1 OF 1

DRAWN BY: C.R.H.
ENGINEER: C. HANDY
PRECISION JOB NUMBER: **1254**
MVE JOB NUMBER: **160011**